

Penn State Health and Human Development Building

State College, PA



Rendering Provided by BCJ

Christopher Graziani
Advisor: Craig Dubler
Construction Management

Penn State Health and Human Development Building

State College, PA



Existing Building Structure

Rendering Provided by BCJ



Previous View From College Avenue

Future View From College Avenue



Rendering Provided by BCJ



Multi-Use Spaces

Rendering Provided by BCJ

Project Cost: \$59 Million
Dates of Construction: February 2013 – June 2015
Occupancy Type: Office/ Lab/ Classroom

Penn State Health and Human Development Building

State College, PA



Rendering Provided by BCJ



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Demolition Phase



Previous View From
College Avenue

Future View From
College Avenue



Rendering Provided by BCJ



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Multi-Use Spaces

Project Cost: \$59 Million
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Penn State Health and Human Development Building

State College, PA



Rendering Provided by BCJ

New Construction



Rendering Provided by BCJ



Previous View From College Avenue

Future View From College Avenue



Rendering Provided by BCJ



Rendering Provided by BCJ

Multi-Use Spaces

Project Cost: \$59 Million

Dates of Construction: February 2013 – June 2015

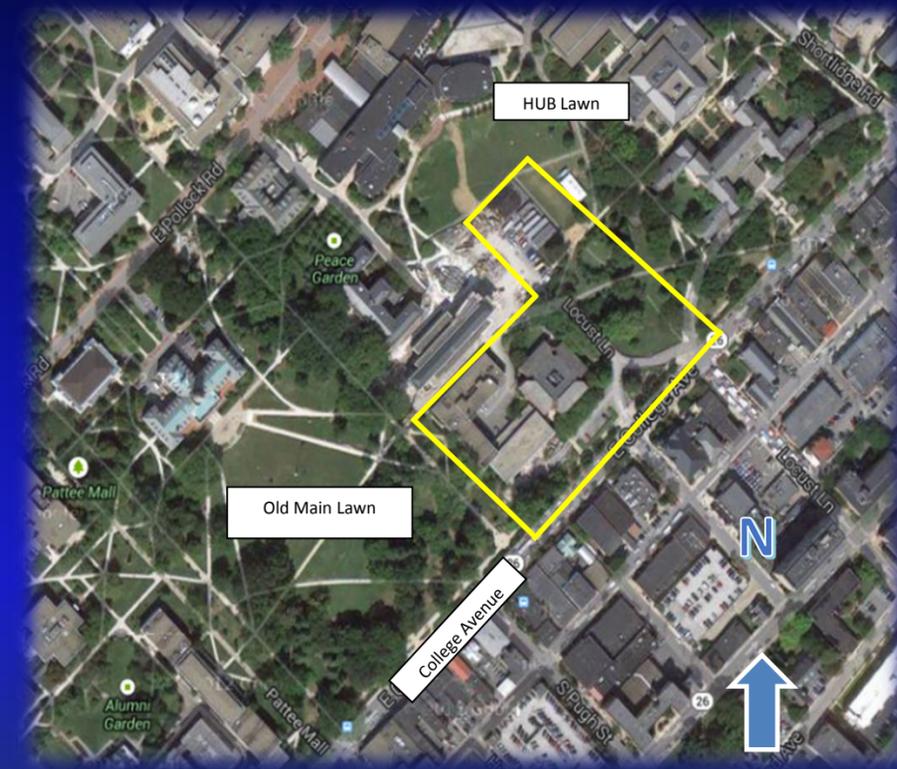
Occupancy Type: Office/ Lab/ Classroom



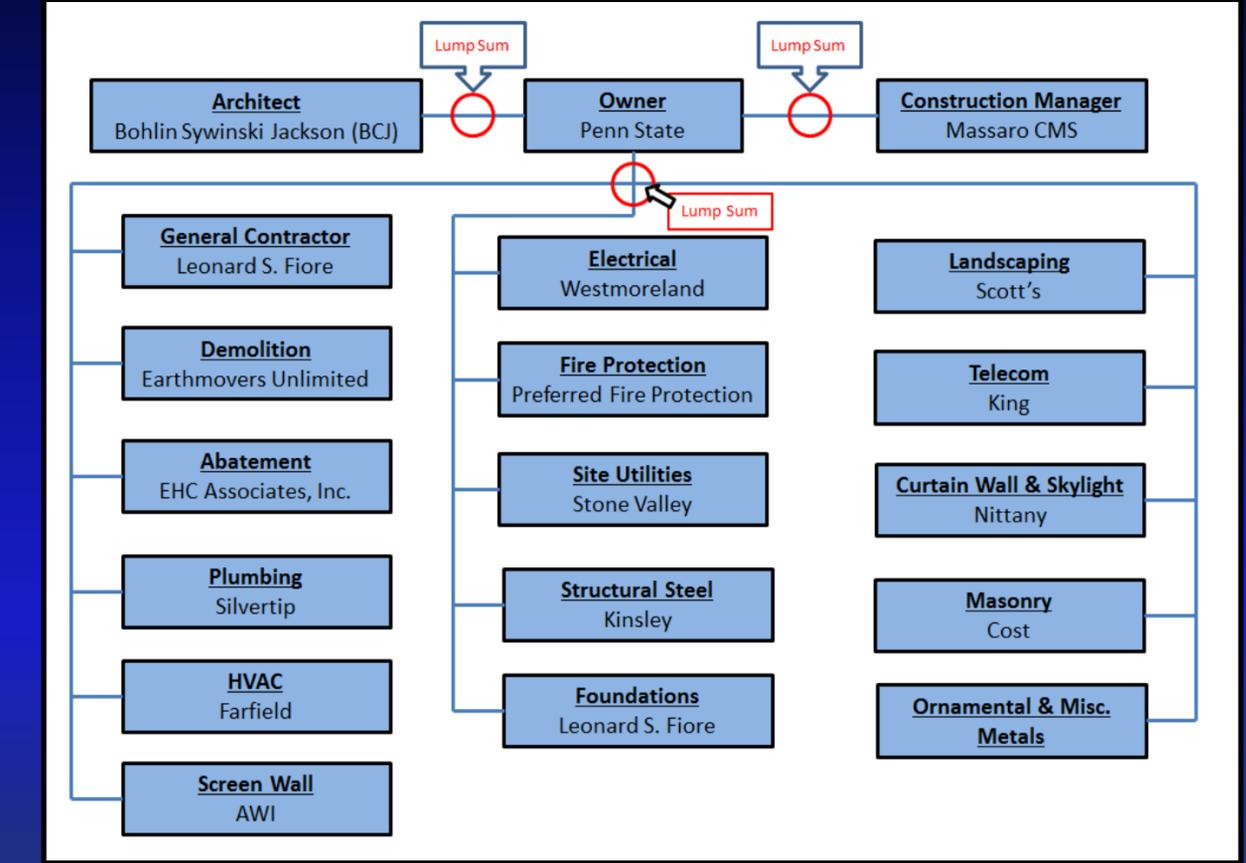
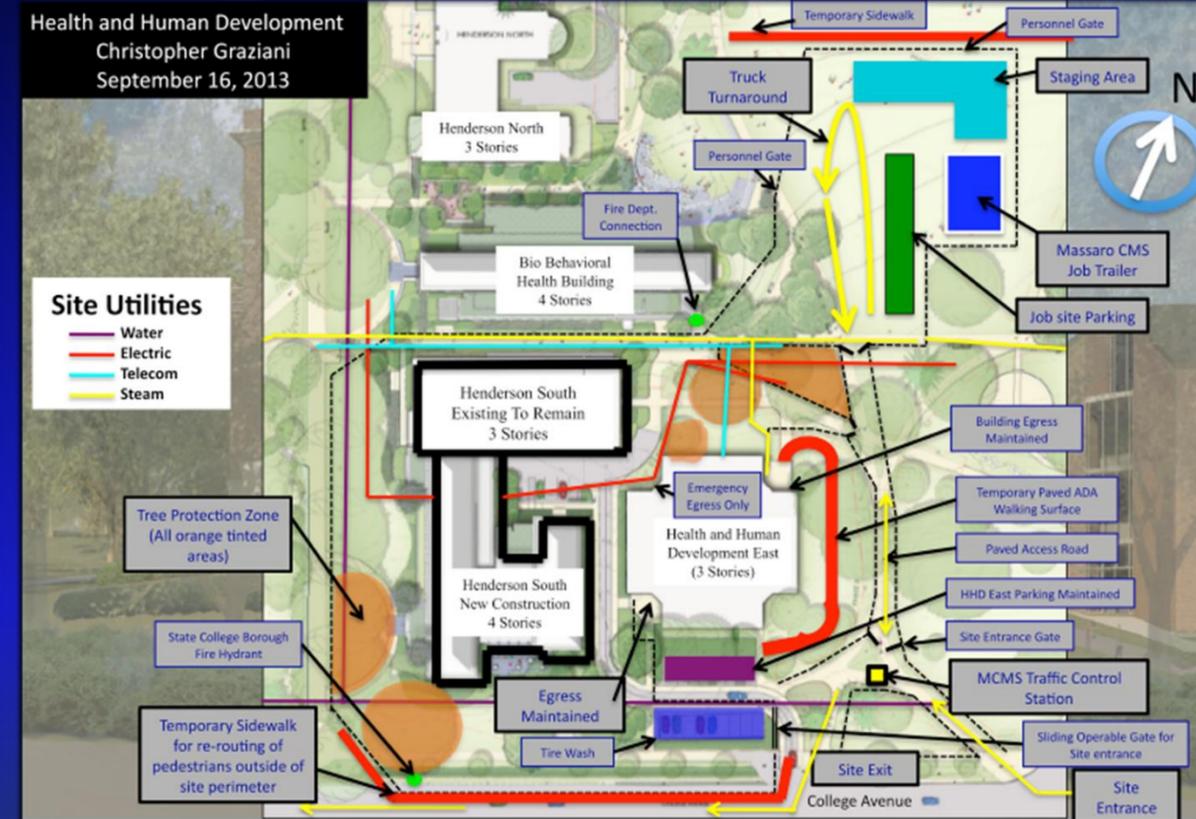
Project Overview



- I. Introduction
 - A. Project Overview
 - B. Project Features
- II. Analysis 1 – Stair Tower Redesign
 - A. Problem Identification & Overview
 - B. Structural Breadth
 - C. Cost and Schedule Impact Analysis
- III. Analysis 2 – Return Air Plenum
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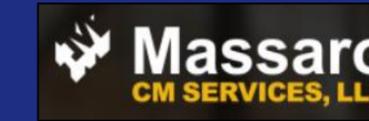


View Taken from Google Maps





Project Features



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Project Features

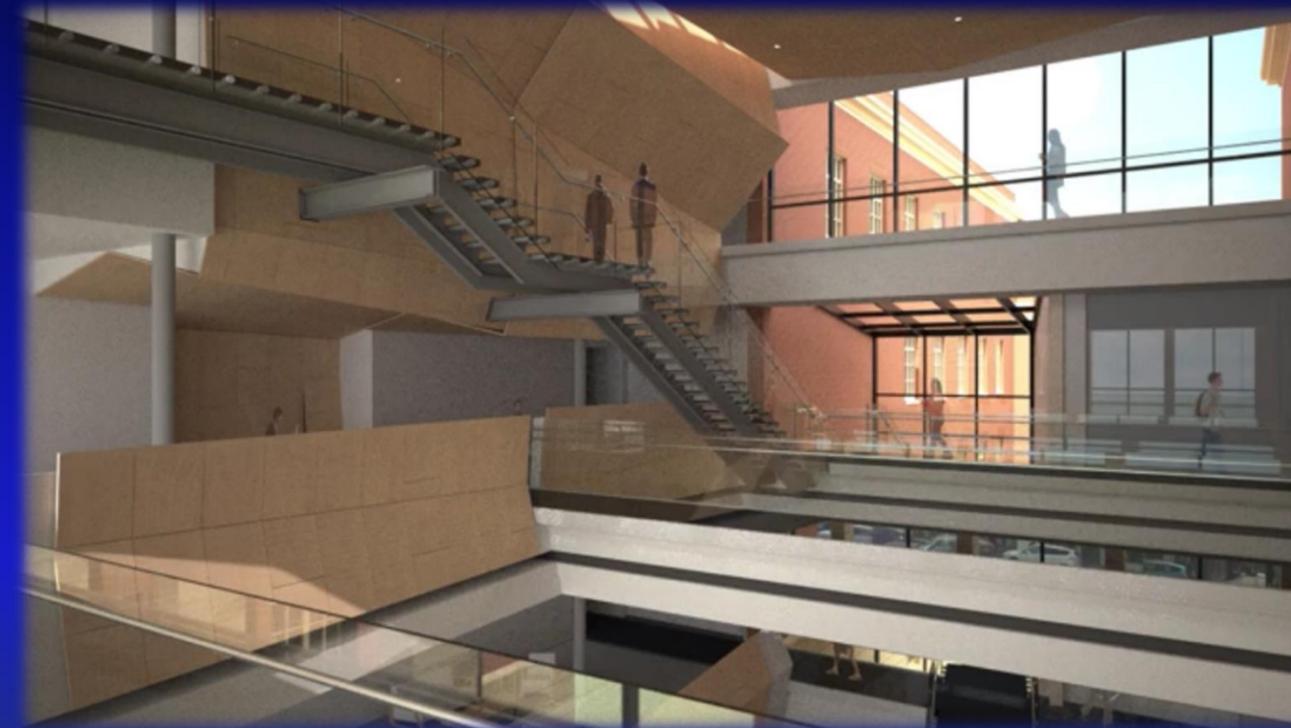
Building Renovation

Large Atrium Space

Architectural Screen Wall

Pursuing LEED Certified

Soil Nail Wall



Rendering Provided by BCJ

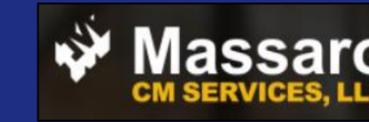
Architectural Screen Wall In Atrium



Soil Nail Wall Installation



Analysis 1 – Stair Tower Redesign



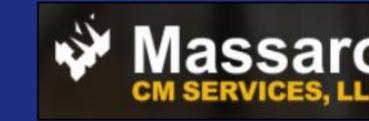
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Problem Statement:

Concrete stair tower acting as a shear wall caused significant delays to the project schedule and provided a low quality product at project turnover



Problem Identification & Overview



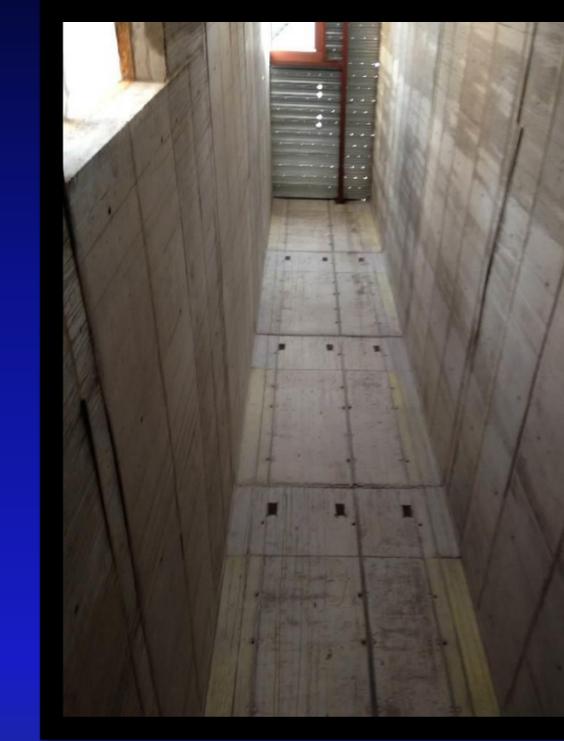
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Desired Benefits

- Faster Installation Time
- Higher Quality of Finish
- Improved Coordination of Trades



Concrete Stair Tower Installation



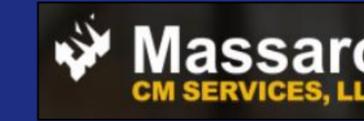
Tower Twisting and Interior Finish



Worker Safety



Structural Breadth



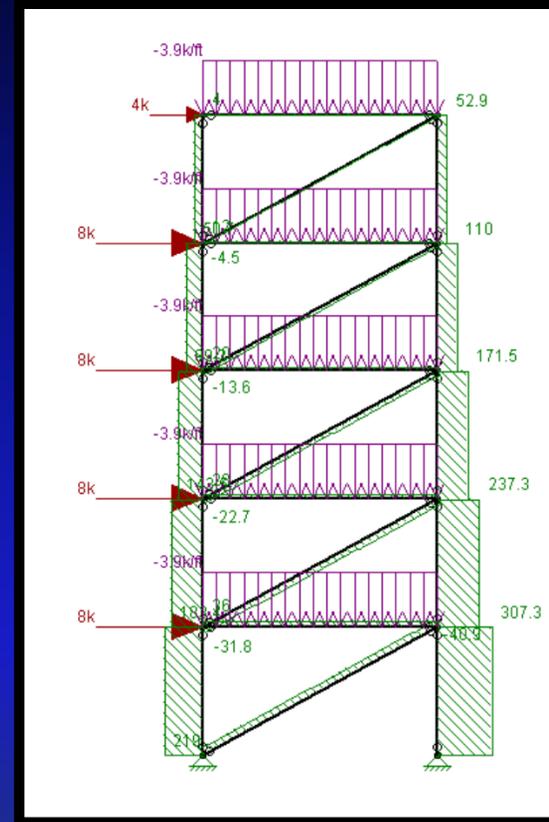
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Purpose:

Change the concrete stair tower to a steel braced frame with drywall infill

Parameters

- Wind Loads
- RISA Program for Calculations
- Fireproofing
- Wall Partition Fill



RISA Program Showing Axial Loads

Maximum Axial Forces
Column = 307.3 Kips
Horizontal Beams = 36 Kips

Steel Design Summary

Size	Quantity	Length (ft)
HSS 4-1/2 x 4-1/2 x 3/8	10	19.8
HSS 6 x 6 x 1/4	10	29.5
W14x90	8	28
W14x90	4	14
W10x30	10	26
W10x17	10	14



Cost and Schedule Impact Analysis



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Interior Finish of Concrete Stair Tower



Photo Taken by David Walenga

Stud Framing Infill of Steel Braced Frame System

Summary for Current Design	
Material	\$ 147,031.50
Equipment	\$ 40,750.00
Manpower	\$ 57,029.20
	\$ 244,810.70
Schedule	5 weeks (25 Days)

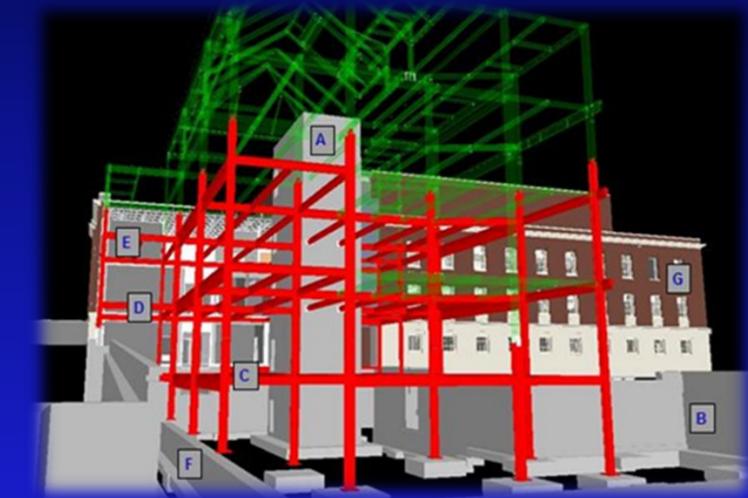
Summary for New Design	
Item	Cost
Steel	\$ 75,955.44
Fireproofing	\$ 7,939.27
Fill (Framed Drywall)	\$ 39,517.70
	\$ 123,412.41
Schedule Summary	
Item	Duration (Days)
Steel Members	1.16
Fireproofing	4.34
6" GWB Partition	20

Cost Savings from Steel Design

- Current Design = \$244,180.70
- Proposed Design = \$123,412.41
 - Savings = \$121,398.29

Schedule Savings from Steel Design

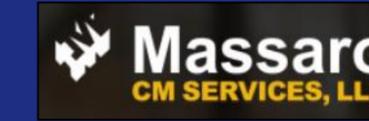
- Current Design = 25 Days
- Proposed Design = 25 Days
 - Savings = 0 Total Days
- **Critical Time Savings = 24 Days**



Stair Tower Construction Compared to BIM Model



Analysis 2 – Return Air Plenum



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Problem Statement:

Tight ceiling spaces cause added schedule time
and heavy coordination of trades is required



System Research



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Purpose:

Utilize a Return Air Plenum System as Opposed to the Return Air Ductwork

Parameters

- Return Air Ductwork Removal
- Fire Damper Requirements
- Plenum Rating Materials
- System Requirements

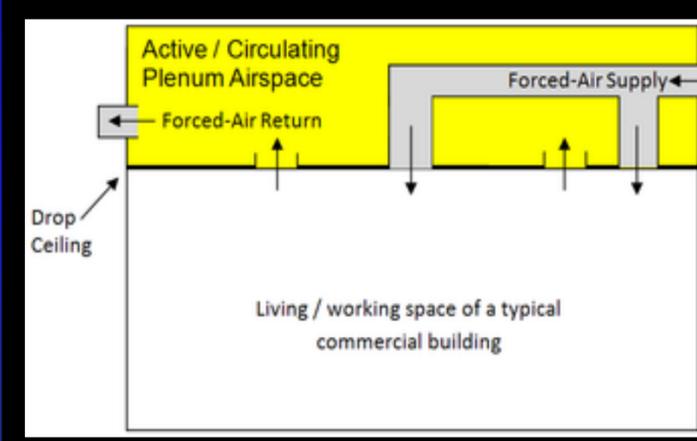
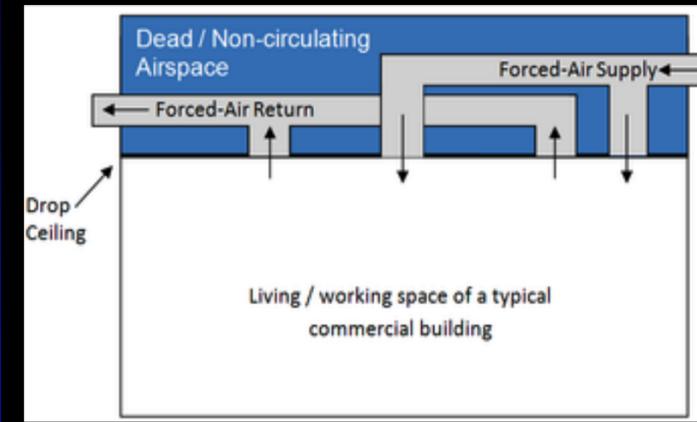


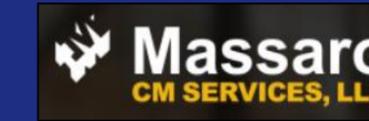
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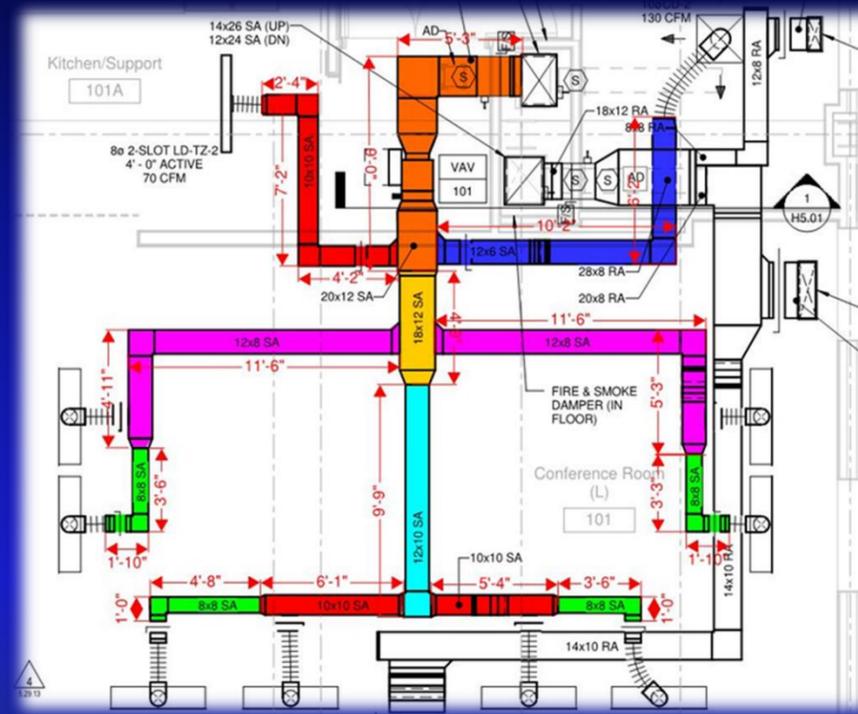
BIM Model Showing Ceiling Space Systems



Cost and Schedule Comparisons



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Supply Air Takeoff for Area Analyzed

Current Design	
Item	Cost
Ductwork	\$ 2,085.70
Fire Dampers (1)	\$ 350.00
	\$ 2,435.70
PLENUM DESIGN	
Item	Additional Cost
Ductwork Insulation Wrap	\$ 948.10

Savings in Area	SF of Area	Savings/SF
\$ 1,487.61	1250	\$ 1.19
Total Area of Building	Savings/SF	Total Savings
150000	\$ 1.19	\$ 178,512.94

Cost Savings from Plenum Design

- Current Design = \$2,435.70
- Proposed Design = \$948.10
- **Savings = \$1,487.61**

Cost Savings from Plenum Design

- Savings per SF of Area = \$1.19
- Total Area of Building = 150,000
- **Total Potential Savings = \$178,512.94**

Schedule Information For Current Design	
Activity	Duration
Prefabrication	2 hours
Raise and Hang	195 minutes
Install Piece that Penetrates Wall	30 minutes
Seal Duct Flange Between Pieces	13 minutes
Insulate Ductwork	63 minutes
Install Fire/Smoke Damper	10 minutes
Total	7 hours

Insulation Wrap = 1 Hour

Savings in Area (Hours)	SF of Area	Savings/SF
6	1250	0.0048
Total Area of Building	Savings/SF	Total Savings
150000	0.0048	720.00



Mechanical Breadth and Conclusions



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Noise Criteria	Air Velocity at Supply Register (fpm)	Air Velocity at Return Grille (fpm)
NC-15 to NC-20	250 to 300	300 to 360
NC-20 to NC-25	300 to 350	360 to 420
NC-25 to NC-30	350 to 425	420 to 510
NC-30 to NC-35	425 to 500	510 to 600
NC-35 to NC-40	500 to 575	600 to 690
NC-40 to NC-45	575 to 650	690 to 780

	ACF 8	
Level	Supply Air Provided (CFM)	Return Air (CFM)
3	2700	3000
3.1	1440	1440
3.2	740	740
3.3	960	960
3.4	1390	1390
	7230	7530

	Airflow (CFM)	Duct Size (in.)	Velocity (FPM)	Friction (in. wg/100')
Air	1440	21	600	0.0245
	680 (l/s)	533 (mm)	3.05 (m/s)	0.2 (Pa/m)

Equivalent Duct Sizes	
Round (in.)	21ø
Rect, most square (1:1 ratio) (in.)	20X20
Rect, flattest (4:1 ratio) (in.)	34X12
Rect, enter one side (in.)	
Rect, duct size (in.)	
Oval, balanced (2:1 ratio) (in.)	ov 30X14
Oval, flattest (4:1 ratio) (in.)	ov 34X12
Oval, enter one side (in.)	
Oval, duct size (in.)	

Settings	
Duct Sizing Increment	2 in.
Rounding	Round Up
Duct Material	Galvanized
Absolute Roughness Factor (ε)	0.0005
Air Temperature (deg. F)	70
Altitude (ft.)	0
Air Density (pcf)	0.075

Duct Calculator Results Provided by KLING STUBBINS

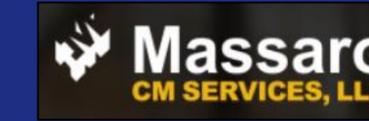
- Amount of Space in the Plenum = 31-3/4"
- Most Square Piece Necessary = 20"x20"
- Flattest Piece Necessary = 34"x12"

Analysis Summary

↓	Schedule Time	6 hours in Area
↓	Cost	\$1,497.61
↑	Coordination	



Analysis 3 – Alternative Excavation Options



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Problem Statement:

Soil is made of dolomite rock so rock excavation techniques are required



Blasting vs. Rock Hammering



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Purpose:

Compare the Cost and Schedule Implications of Rock Excavation Techniques and Research Additional Tactics

Parameters

- Personnel Requirements
- Equipment Necessary
- Soil Classification
- Noise Levels
- Safety



Image Taken from Google Image Search

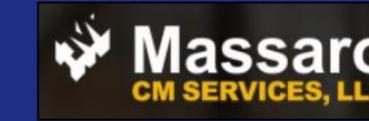
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Traditional Rock Hammering



Blasting vs. Rock Hammering



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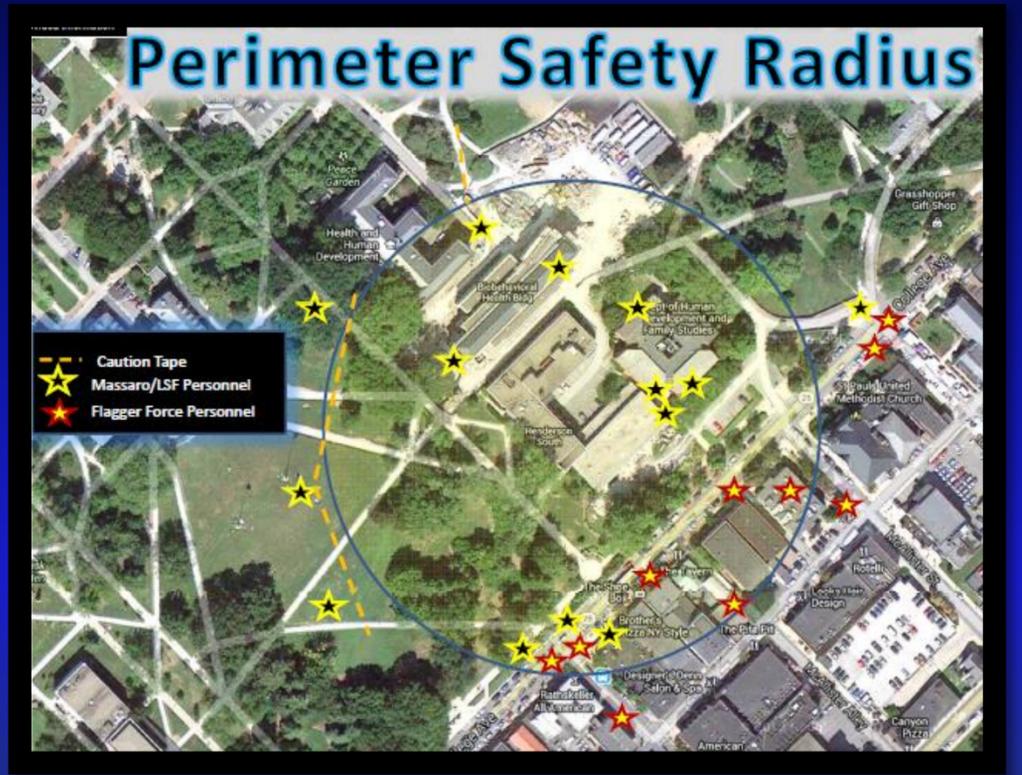
Traditional Rock Hammering



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Traditional Excavation Takeoff	
Unrippable Soil	
Cost/CY	Cost
\$ 50.00	\$ 186,825.00
Rippable Soil	
Cost/Cy	Cost
\$ 25.00	\$ 93,412.50
Total	\$ 280,237.50
Additional Costs	
General Conditions	\$ 5,000.00
Subtotal	\$ 285,237.50
Shoring Requirements	\$ 400,000.00
Total	\$ 685,237.50

Blasting Totals	
Blasting Estimate	\$ 95,000.00
Manpower Takeoffs	\$ 74,954.96
Excavation	\$ 186,825.00
Total	\$ 356,779.96

Cost Savings from Blasting
\$328,457.54

Blasting Schedule Time

- Blasting = 14 Days
- Excavation = 8 Days
- Total Time = 22 Days

Traditional Schedule Time

- Unrippable Soil = 12 Days
- Rippable Soil = 15 Days
- Total Time = 27 Days

Time Savings From Blasting
5 Days



Research and Conclusions



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Blasting



- Schedule
- Cost
- Safety
- Noise

Traditional



Image Taken from Google Image Search

Royex Technology

- Minimal Fly Rock
- Minimal Rock Vibrations
- Lower Noise Levels
- Faster
- Minimal Personnel Required
- Safer to Transport
- Cost Savings

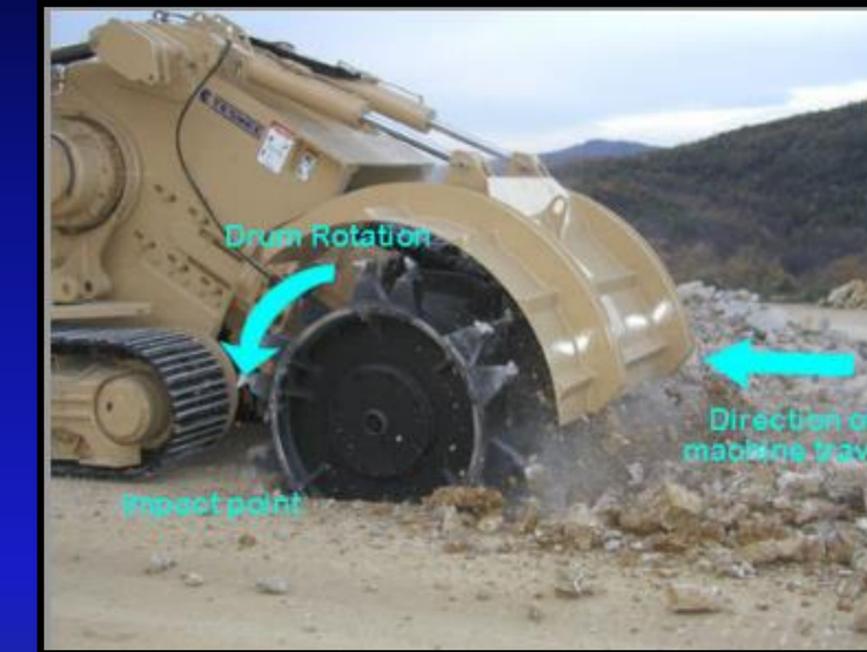


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Rock Hawk

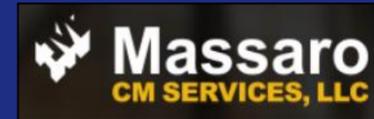
- 110 ton, 630 hp Machine
- Top-Down Cutting Technology
- Material Recycled for Backfill



Image Taken from Google Image Search



Conclusions & Recommendations



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Analysis 1 – Stair Tower Redesign

- Potential cost savings of \$121,398.29
- Critical schedule savings of 24 days
- Improved quality and safety
- More logical to use steel braced frame on future projects

-  Improves Quality
-  Accelerates Schedule
-  Improves Safety

Analysis 2 – Return Air Plenum

- Potential cost savings of \$1,497.61 in area
- Schedule savings of 6 hours in area
- Reduce amount of coordination necessary in ceiling space
- Industry standard to plenum rate materials → return air plenum is more logical

Analysis 3 – Alternative Excavation Options

- Blasting was cheaper due to lack of shoring necessary
- Blasting saved an estimated 5 days of schedule time
- Traditional rock hammering is a major noise disruption
- If the schedule allows, blasting is preferred



Image Taken from Google Image Search





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Academic Acknowledgements

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Craig Dubler – CM Advisor
Moses Ling – Mechanical Advisor



Industrial Acknowledgements

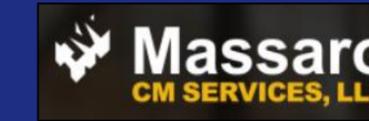


Special Thanks

The Massaro Project Team
Tim Jones – Massaro Project Manager
Kevin Nestor – Massaro Senior Project Manager
Keith Smith – Massaro Site Manager
Dan Kiefer – Massaro Senior Estimator
David Walenga – Ruby + Associates
Geoff Measel – G.E.M. Construction
Family and Friends



Questions?

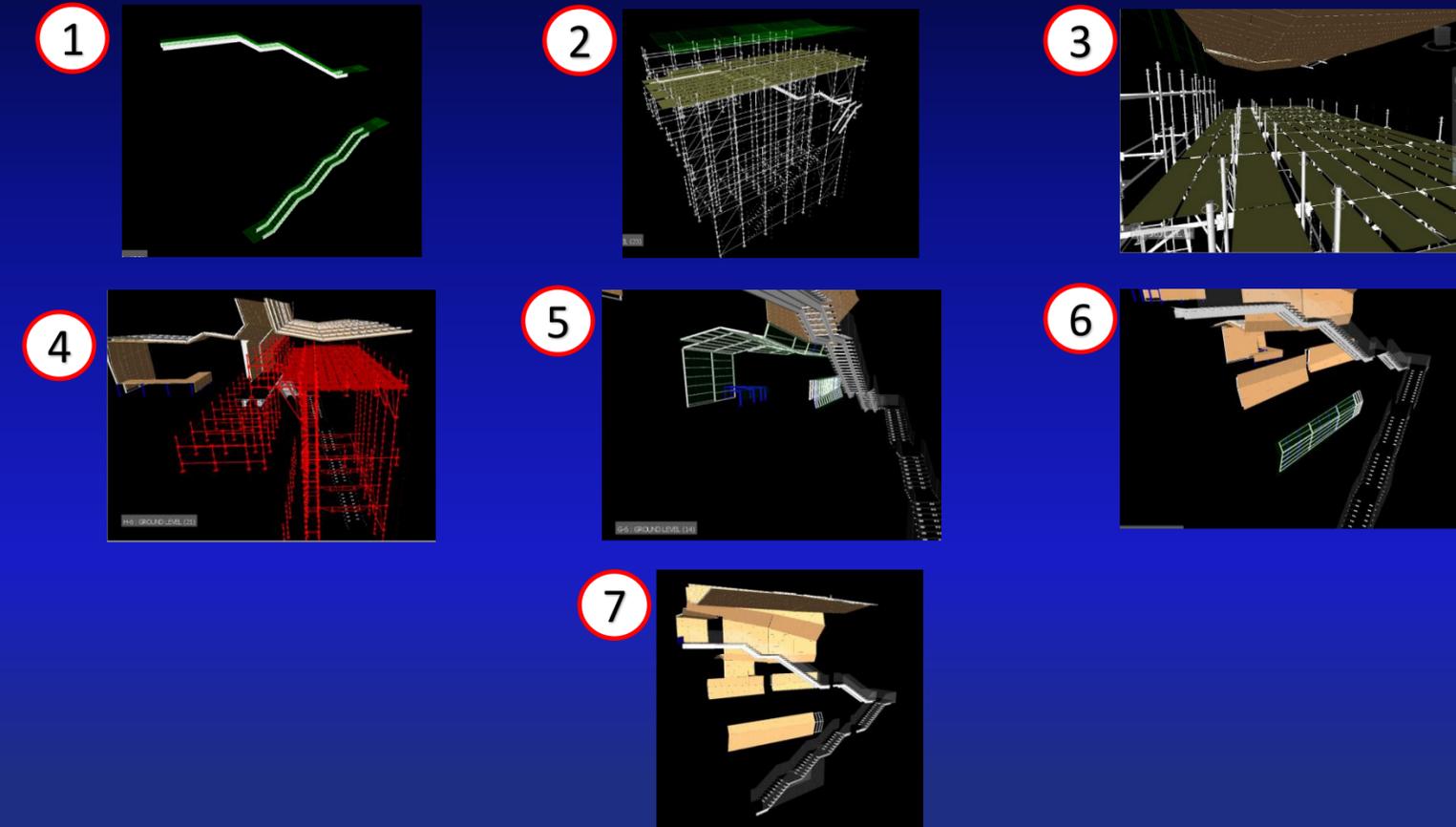


Rendering Provided by BCJ



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Analysis 4 – Re-Sequencing of Atrium Systems





Appendix



Current Stair Tower Design

STAIR A: LEVEL 1 - TOP OF STRUCTURE					Unit	\$ per unit	Total \$
		12	" THICK	4,690	SF		
CONCRETE - BUY				182	CY	125.00	\$22,799
REBAR		50	#/CY	9,119	LBS	2.00	\$18,239
FORMING				9,380	SF	10.00	\$93,800
RUBBING				9,380	SF	1.30	\$12,194
Stair A			- COST PER CUBIC YARD		\$806	/CY	
Stair A			- COST PER SQUARE FOOT		\$31	/SF	
CIP WALL @ STAIR A			-SUBTOTAL	\$147,032			

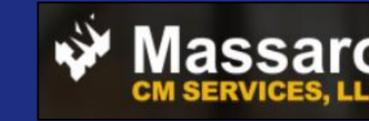
Equipment Takeoff for Current Design					
Type of Equipment	Days/Level	# of Levels	Total Number of Days	Cost/Day	Total Cost
Crane	5	5	25	\$ 1,250.00	\$ 31,250.00
Lift	5	5	25	\$ 180.00	\$ 4,500.00
Pump Truck	1	5	5	\$ 1,000.00	\$ 5,000.00
					\$ 40,750.00

Manpower Takeoff for Current Design						
Type of laborer	Number of Workers	Hours Per Worker Per Level	# of Levels	Total Number of Hours	Cost/Hour	Total Cost
Iron Workers	2	12	5	120	\$ 49.63	\$ 5,955.60
Carpenter	3	40	5	600	\$ 38.60	\$ 23,160.00
Laborer	3	40	5	600	\$ 29.14	\$ 17,484.00
Crane Operator	1	40	5	200	\$ 43.54	\$ 8,708.00
Pump Operator	1	8	5	40	\$ 43.04	\$ 1,721.60
						\$ 57,029.20

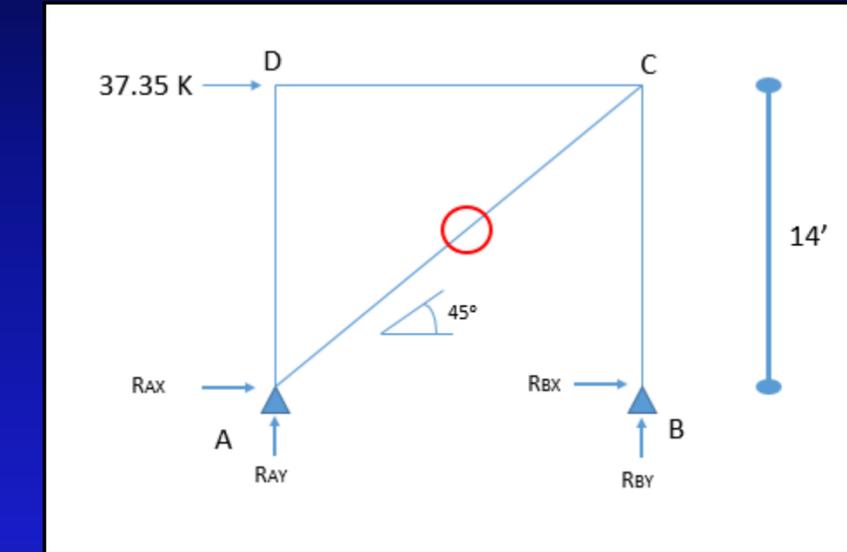
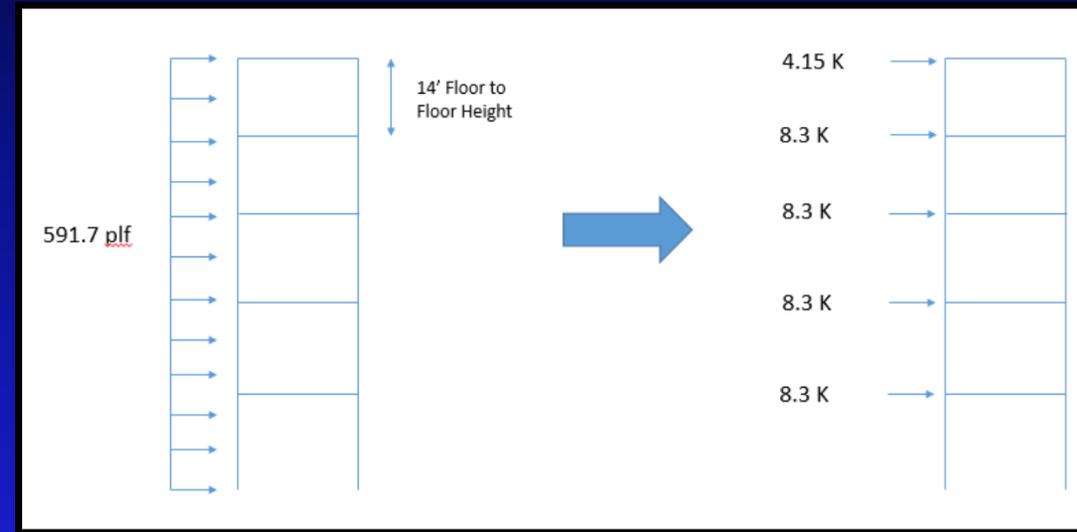
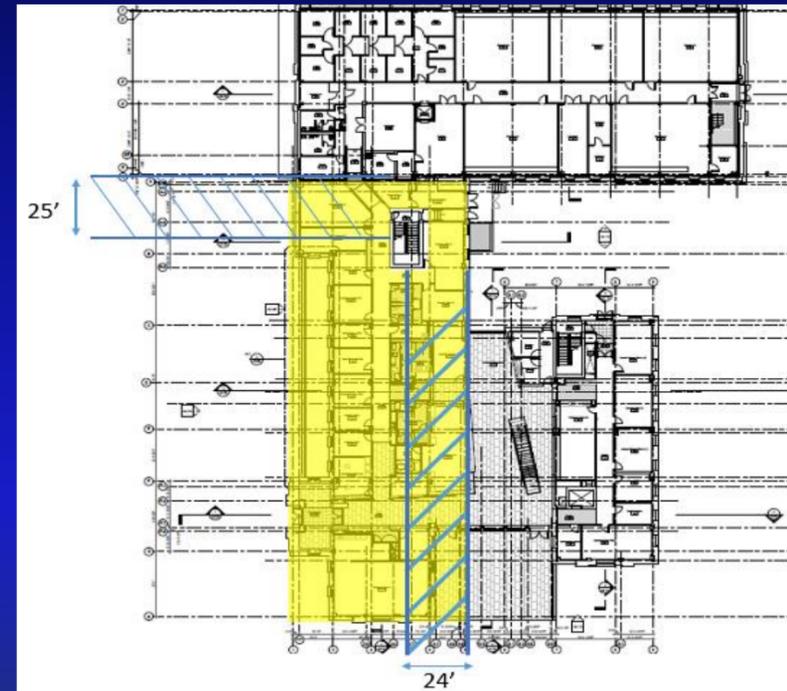
Summary for Current Design	
Material	\$ 147,031.50
Equipment	\$ 40,750.00
Manpower	\$ 57,029.20
Total	\$ 244,810.70
Schedule Time	5 Weeks



Appendix

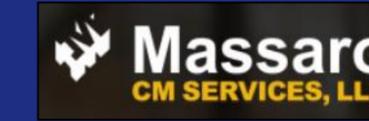


Structural Breadth Calculations

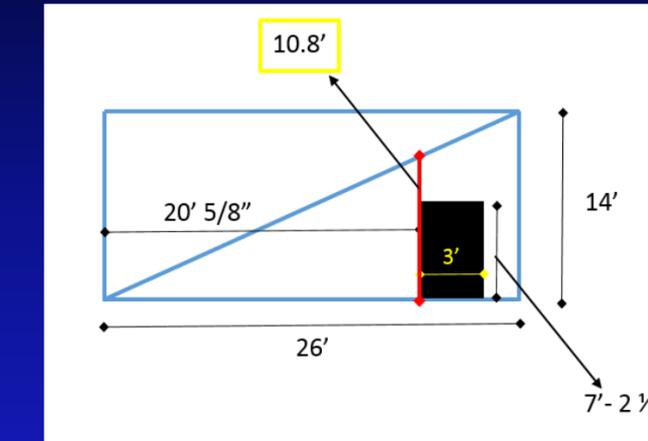
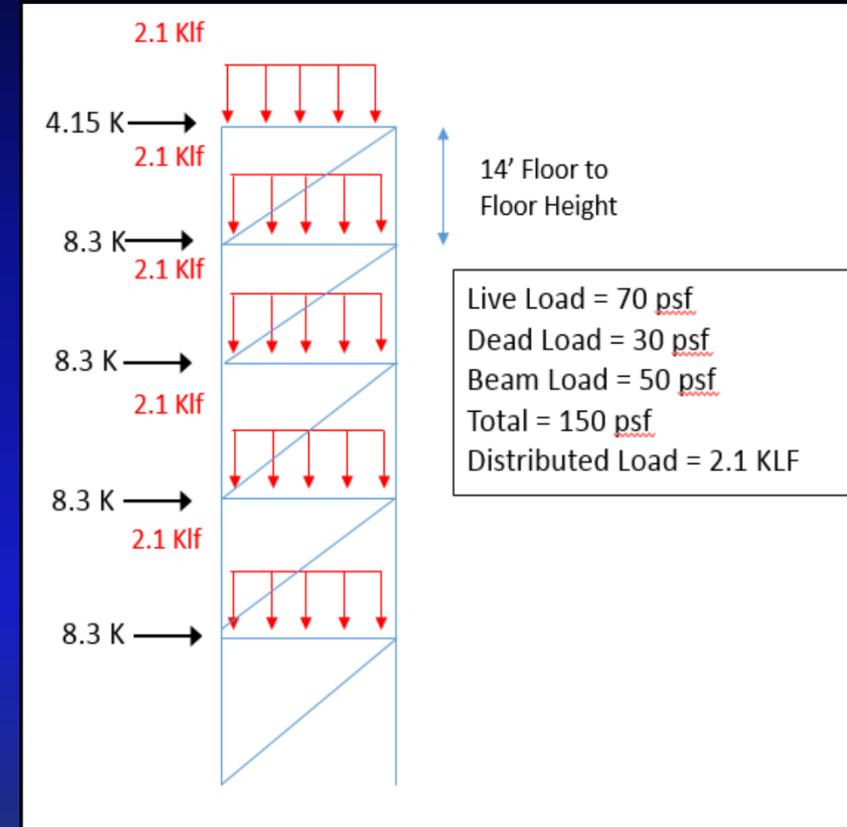
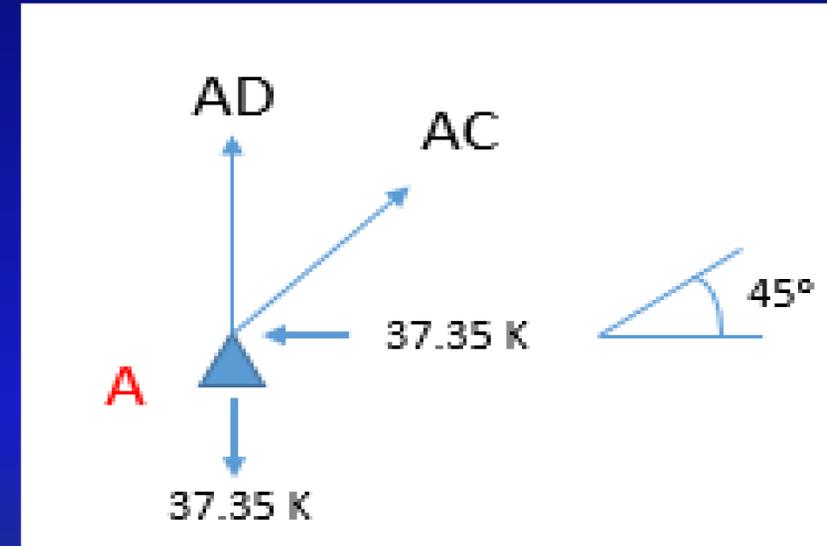




Appendix



Structural Breadth Calculations





Appendix



Steel Design Summary												
Size	Quantity	Length (ft)	Sum of Lengths	LB/LF	Lbs	Tons	Material Cost/LF	Labor Cost/ LF	Equipment Cost/ LF	Total/ LF	Cost/Ton	Total Cost
HSS 4-1/2 x 4-1/2 x 3/8	10	19.8	198	19.82	3924.36	1.96					\$ 3,000.00	\$ 5,886.54
HSS 6 x 6 x 1/4	10	29.5	295	19.02	5610.9	2.81					\$ 3,000.00	\$ 8,416.35
W14x90	8	28	224	90	20160	10.08	\$ 172.00	\$ 2.86	1.59	\$ 176.45		\$ 39,524.80
W14x90	4	14	56	90	5040	2.52	\$ 172.00	\$ 2.86	1.59	\$ 176.45		\$ 9,881.20
W10x30	10	26	260	30	7800	3.9	\$ 47.00	\$ 4.99	2.77	\$ 54.76		\$ 14,237.60
W10x17	10	14	140	17	2380	1.19	\$ 31.50	\$ 4.58	2.54	\$ 38.62		\$ 5,406.80
												\$ 69,050.40
											Add 10%	\$ 6,905.04
												\$ 75,955.44

Fireproofing											
Size	Quantity	Length (ft)	Sum of Lengths	Surface Length	Square Footage	Material	Labor	Equipment	Total	Inches	Total Cost
HSS 4-1/2 x 4-1/2 x 3/8	10	19.8	198	1.5	297	\$ 0.53	\$ 0.60	\$ 0.09	\$ 1.22	2.00	\$ 724.68
HSS 6 x 6 x 1/4	10	29.5	295	2	590	\$ 0.53	\$ 0.60	\$ 0.09	\$ 1.22	2.00	\$ 1,439.60
W14x90	8	28	224	4.8	1065.49	\$ 0.53	\$ 0.60	\$ 0.09	\$ 1.22	2.00	\$ 2,599.80
W14x90	4	14	56	4.8	266.37	\$ 0.53	\$ 0.60	\$ 0.09	\$ 1.22	2.00	\$ 649.95
W10x30	10	26	260	2.7	705.47	\$ 0.53	\$ 0.60	\$ 0.09	\$ 1.22	2.00	\$ 1,721.34
W10x17	10	14	140	2.4	329.47	\$ 0.53	\$ 0.60	\$ 0.09	\$ 1.22	2.00	\$ 803.90
											\$ 7,939.27

Material				
Description	Quantity	Unit	Material Unit Cost	Total Cost
6" Studs @ 16" O.C.	400	LF	30	\$ 12,000.00
5/8" Drywall - Taped and Finsihed	11200	SF	1.52	\$ 17,024.00
Joint Sealant	400	LF	0.3	\$ 120.00
Sound Attenuation Blanket	4000	SF	0.44	\$ 1,760.00
				\$ 30,904.00

Labor				
Type of Manpower	Quantity	Unit	Cost per Unit	Total Cost
Carpenter & Taper	20	MD	357.04	\$ 7,143.02
Laborer	3.3	MD	273.12	\$ 910.68
Laborer (stocking)	11200	SF	0.05	\$ 560.00
				\$ 8,613.70

Schedule							
Steel Members							
Size	Quantity	Length (ft)	Sum of Lengths	Tons	Tons/Day	Daily Output/LF	Days
HSS 4-1/2 x 4-1/2 x 3/8	10	19.8	198	1.96	30		0.07
HSS 6 x 6 x 1/4	10	29.5	295	2.81	30		0.09
W14x90	8	28	224	10.08		960	0.23
W14x90	4	14	56	2.52		960	0.06
W10x30	10	26	260	3.9		550	0.47
W10x17	10	14	140	1.19		600	0.23
							1.16

Fireproofing		
Daily Output/SF	Total SF	Total Days
1500	3253.8	4.34

6" GWB Partition Material					
Description	Quantity	Unit	Production Rate Per day	Unit	Total Time Needed (Days)
6" Studs @ 16" O.C.	400	LF	100	LF	4
5/8" Drywall - Taped and Finsihed	11200	SF	965	SF	11.61
Joint Sealant	400	LF	1000	LF	0.4
Sound Attenuation Blanket	4000	SF	1000	SF	4
					20



Appendix



Check Steel Manual
Tension →
Compression → Based on length of member

100 psf load → Multiply it by the span (14') divided by 1000 → = 2.1 k/ft

19.799'

Check the 53k Axial Force in Compression
HSS 4 1/2 x 4 1/2 x 3/8 → 59.9k ✓

CHECK IN TENSION → 227k Yielding 179k Rupture ✓
Use HSS 4 1/2 x 4 1/2 x 3/8 FOR ALL CROSS BRACES

- Put the Model into Risa
- ADD LOADS
- GET AXIAL FORCES

* USE THE LARGEST SIZE THE WHOLE WAY UP. LARGEST MEMBERS ON THE BOTTOM

Use 28' Long For Columns as Maximum Length
Want to use W14 for Column sizing because they splice well together.
Compression → 215 kips ✓
Tension → 806 Yielding ✓
653 Rupture ✓

Use W10x17 For Beams
Max. load = 40.1k ✓

19.642 psf x 28' = 491.05 plf
19.642 psf x 24' = 471.05 plf

3.5k
6k
6k
6k
6k

Live Load = 70 psf
Dead Load = Assumed 80 psf
Assume Load of beams = 50 psf

Bottom Frame in Elevation View

$\Sigma F_x = 0 = 37.35 + R_{Ax} = 0$
 $R_{Ax} = -37.35$ $R_{Ax} = 37.35 \leftarrow$

$\Sigma F_y = 0 = R_{Ay} + R_{By} = 0$
 $R_{Ay} = -37.35$ $R_{Ay} = 37.35 \downarrow$

$\Sigma M_A = 0 \Rightarrow R_{By} (14') = 37.35 (14')$
 $R_{By} = 37.35 \uparrow$

$\Sigma F_y = -37.35 + AD_y + AC \sin(45) = 0$
 $\Sigma F_x = -37.35 + AC \cos(45) = 0$
 $AC \cos(45) = 37.35$
 $AC = 53k$

Don't account for Dead & Live load for this diag. in only 100 psf lateral.

HSS Steel

Other side of Stair Tower
Wind Load = 23.668 psf
23.668 psf x 24' = 568.032 plf

4.0k
8.0k
8.0k
8.0k
8.0k

Σ Horizontal Forces = 36.0k

Angle: $\tan(\text{Angle}) = \frac{14}{26}$
Angle = 28.3°

$\Sigma F_x = 0 = 36 + R_{Ax} = 0$
 $R_{Ax} = -36k$ $R_{Ax} = 36k \leftarrow$

$\Sigma F_y = 0 = R_{Ay} + R_{By} = 0$
 $R_{Ay} = -19.4k$ $R_{Ay} = 19.4k \downarrow$

$\Sigma M_A = 0 \Rightarrow R_{By} (26') = 36 (14')$
 $R_{By} = 19.4k \uparrow$

$\Sigma F_y = -19.4k + AD_y + AC \sin(28.3) = 0$
 $\Sigma F_x = -36 + AC \cos(28.3) = 0$
 $AC \cos(28.3) = 36 \Rightarrow AC = 40.89k$

CHECK STEEL MANUAL
• IDEAL FROM A CONSTRUCTION STANDPOINT TO USE SAME SIZE
→ CHECK COMPRESSION FIRST SINCE IT CONTROLS

26'
14'

SAME SIZE WILL NOT WORK FOR COMPRESSION

USE HSS 6x6 x 1/4 FOR CROSS BRACES
→ COMPRESSION FOR 30' LENGTH = 50.0 k ✓
→ TENSION → YIELDING = 217k ✓ RUPTURE = 171k ✓

Each Floor
3,916L
180 psf Load
Span of 26'

Columns # Use 28' Long For Columns as Maximum Length
Want to use W14 for Column sizing because they splice well together.
Try W14x90
Compression → 653 kips ✓
Tension → 1190 Yielding ✓
970 Rupture ✓

Beams
Span = 26'
can use a W10x30
→ Max. Load = 42.2k



Appendix



RA Ductwork Takeoffs									
Size	Sum of the two sides	Max Dimension	Gage	lb/ft	Length (ft)	Pounds	SF	Cost/pound	Cost
18x12	30	18 -> 30	24	6.5	3	19.50	13.86913	\$ 8.50	\$ 117.89
28x8	36	28 -> 30	24	7.8	7	54.60	38.83357	\$ 8.50	\$ 330.09
8X8	16	8 -> 30	24	3.4	3	10.20	7.254623	\$ 8.50	\$ 61.66
12X8	20	12 -> 30	24	4.3	7	30.10	21.40825	\$ 8.50	\$ 181.97
16X6	22	16 -> 30	24	4.7	2	9.40	6.685633	\$ 8.50	\$ 56.83
20X8	28	20 -> 30	24	6	10	60.00	42.67425	\$ 8.50	\$ 362.73
14X10	24	14 -> 30	24	5.2	31	161.20	114.6515	\$ 8.50	\$ 974.54
TOTALS						345.00	245.377		\$ 2,085.70

Supply Air Takeoff							
Size	Sum of the two sides	Max Dimension	Gage	lb/ft	Length (ft)	Pounds	SF
20x12	32	20 --> 30	24	6.9	14	98.3	85.1
10x10	20	10 --> 30	24	4.3	25	107.9	93.3
12x6	18	12 --> 30	24	3.9	16	63.7	55.1
18x12	30	18 --> 30	24	6.5	5	30.9	26.7
12x8	20	12 --> 30	24	4.3	33	142.6	123.4
8x8	16	8 --> 30	24	3.4	21	70.0	60.5
12x10	22	12 --> 30	24	4.7	10	45.8	39.6
							483.7

SCHEDULE INFORMATION			
PREFABRICATION - 1 hr labor per 200 lbs of sheetmetal			
2	hrs total		
15 MINUTES TO RAISE AND HANG ONE RECTANGULAR PIECE OF DUCTWORK			
13	Pieces	15	minutes/piece 195
ADD 15 MINUTES OF INSTALL TIME FOR EACH PIECE OF DUCT WHICH PENETRATES A WALL			
2	Pieces that Penetrate	15	minutes/piece 30
1 MINUTE TO SEAL 24 LINEAR INCHES OF DUCT FLANGE BETWEEN PIECES			
13	Areas to sea	1	Minute/seal 13
10 MINUTES TO INSULATE 10 LINEAR FEET OF DUCTWORK			
63	LF	10	Minutes/10 LF 63
			404.5
			6.7 Hours

Items in the Ceiling	Does it need to be Plenum Rated?	Is it already Plenum Rated?
Steel	No	-
Electrical Conduit	Yes	Yes
Cables	Yes	Yes
SA Ductwork	Insulation needs wrapped	-
Sprinkler System	no	-

* Requirements say that the insulation wrap needs to be 1/2 inch thick					
According to Subcontractor	Cost per SF	SF	Total Cost	Minutes/SF	Minutes
1" Vapor Barrier Wrap	1.96	484	\$ 948.10	0.15	73



Appendix



Blasting Estimate	
Blasting	\$55,000.00
Pre Blast Survey	\$20,000.00
Monitors	\$ 6,000.00
Blasting Mats	\$ 6,000.00
Stone	\$ 1,000.00
Mobilization	\$ 7,000.00
Total	\$95,000.00

Manpower Takeoffs					
Type of Personnel	Hours/day	Number of days	Total Number of Hours	Cost per Hour	Total Cost
Project Manager	2	14	28	\$ 95.00	\$ 2,660.00
Site Managers (3)	6	14	84	\$ 85.00	\$ 7,140.00
Superintendent	2	14	28	\$ 100.00	\$ 2,800.00
Senior PM	2	14	28	\$ 100.00	\$ 2,800.00
Intern (2)	4	14	56	\$ 35.00	\$ 1,960.00
Project Engineer (3)	6	14	84	\$ 75.00	\$ 6,300.00
LSF Laborers (6)	12	14	168	\$ 29.14	\$ 4,895.52
Douglas Blaster In Charge	8	14	112	\$ 45.19	\$ 5,061.28
Douglas Personnel	8	14	112	\$ 45.19	\$ 5,061.28
Flagger Force (10)	45	14	630	\$ 39.00	\$ 24,570.00
Excavation Operators (2)	16	17	272	\$ 43.04	\$ 11,706.88
					\$ 74,954.96

Excavation Takeoff For Blasting		
CY needed to be Excavated	Cost/ CY	Total Cost
7473	\$ 25.00	\$186,825.00



Appendix



Traditional Excavation Takeoff			
Assumed half unrippable material and half rippable because dolemite rock works like a sin graph			
Unrippable Soil			
Cost/CY	Cost	cy/day	Days
\$ 50.00	\$ 186,825.00	315	12
Rippable Soil			
Cost/Cy	Cost	CY/Day	Days
25	\$ 93,412.50	500	15
Total	\$ 280,237.50		27
Additional Costs			
General Conditions	\$ 5,000.00		
Subtotal	\$ 285,237.50		
Shoring Requirements	\$ 400,000.00		
Total	\$ 685,237.50		